

# Relating Available Stopping Sight Distance to Crash Risks and Other Insights from NCHRP 15-75

Peter T. Savolainen, Ph.D., P.E.

MSU Research Foundation Professor & Chairperson

Department of Civil & Environmental Engineering

Michigan State University



## Overview

- Introduction, Scope, and Review of Existing Guidance
- Summary of Findings
- Recommended Changes to 2018 AASHTO Green Book
- SSD vs. Crash Risk
- The Future
- Question-and-Answer

## Review of Existing Guidance for Stopping Sight Distance (SSD)



## Stopping Sight Distance (SSD)

- **“The minimum sight distance required for a driver to stop a vehicle after seeing an object in the roadway without hitting the object”**
- In general, enough distance should be provided so that a below average driver can stop.
- Greater lengths may be desirable.





## Why is SSD Important?

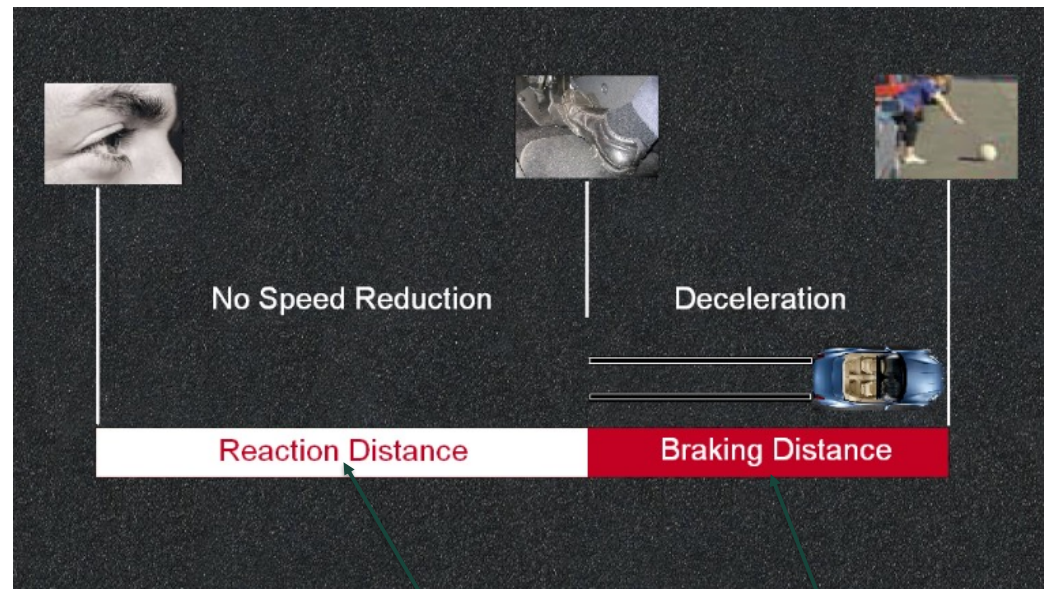
- Highways must be designed so that there is adequate stopping sight distance
- SSD influences:
  - Lengths of crest vertical curves
  - Lengths of sag vertical curves
  - Overpass heights
  - Horizontal offset to objects along horizontal curves



## Study Overview

- The American Association of State Highway and Transportation Officials (AASHTO) *A Policy on Geometric Design of Highways and Streets* (i.e., the “Green Book”) provides guidance for determining geometric design criteria of roadways, including stopping sight distance (SSD).
- SSD is influenced by many factors (e.g., perception reaction time, object height, driver’s eye height). Changes in the driving population, vehicle fleet, and other areas motivate the need to critically assess this guidance, which was last updated in the 2001 edition of the Green Book.

## Stopping Sight Distance (SSD)



$$\text{Distance}_{\text{Stopping}} = \text{Distance}_{\text{Perception/Reaction}} + \text{Distance}_{\text{Braking}}$$

$$SSD = D_r + D_b$$

## AASHTO SSD Model

- $SSD = d_R + d_B$ 
  - Where:
  - $SSD$  = stopping sight distance
  - $d_R$  = reaction distance
  - $d_B$  = braking distance
- $SSD = 1.47Vt + \frac{V^2}{30\left[\left(\frac{a}{32.2}\right) \pm G\right]}$ 
  - Where:
  - $V$  = design speed (mph)
  - $t$  = brake reaction time (s)
  - $a$  = deceleration rate (ft/s<sup>2</sup>)
  - $G$  = grade (ft/ft)

## Evolution of SSD Model

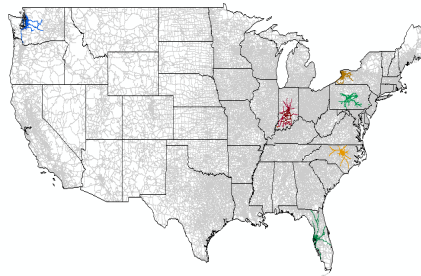
Year	Driver's Eye Height (ft.)	Object Height (ft.)	Brake Reaction Time (sec.)	Vehicle Speed	Pavement Condition	Friction Factors	Deceleration Rate (ft/s <sup>2</sup> )
1928	horizontal and vertical curves be used which provide a sight distance at least 500 feet						
1940	4.5	0.33	3.0 for 30 mph; 2.0 for 70 mph	Design Speed	Dry	0.50 at 30 mph to 0.40 at 70 mph	
1954	4.5	0.33	2.5	Lower than design speed	Wet	0.36 at 30 mph to 0.29 at 70 mph	
1965	3.75	0.5	2.5	Lower than design speed	Wet	0.36 at 30 mph to 0.27 at 70 mph	
1970	3.75	0.5	2.5	Min: lower than design speed; Max: design speed	Wet	0.35 at 30 mph to 0.27 at 70 mph	
1984	3.5	0.5	2.5	Min: lower than design speed; Max: design speed	Wet	0.35 at 30 mph to 0.28 at 70 mph	
1990	3.5	0.5	2.5	Min: lower than design speed; Max: design speed	Wet	0.35 at 30 mph to 0.28 at 70 mph	
1994	3.5	0.5	2.5	Min: lower than design speed; Max: design speed	Wet	0.35 at 30 mph to 0.28 at 70 mph	
2001	3.5	2	2.5	Design Speed	Wet		11.2
2004	3.5	2	2.5	Design Speed	Wet		11.2
2011	3.5	2	2.5	Design Speed	Wet		11.2
2018	3.5	2	2.5	Design Speed	Wet		11.2

## Summary of Brake Reaction Time Research

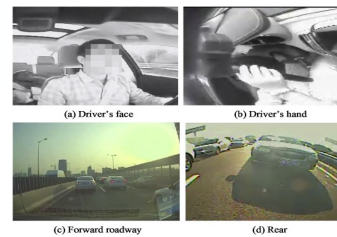
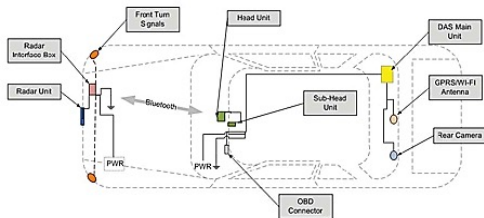
<b>Unsuspecting Driver (Unexpected Event)</b>								
	N	Ages	Distraction-Involved?	Mean (sec.)	Std. Dev (Sec.)	85th Pct. (Sec.)	95th Pct. (Sec.)	Stimulus
<b>Field Collection (Drivers were unaware of being observed)</b>								
Sivak et al., 1982	1,644	Mix	No	1.21	0.63	1.78	2.40	Unexpected signal
Wortman and Matthias, 1983	839	Mix	No	1.30	0.60	1.80	2.35	Unexpected signal
Chang et al., 1985	579	Mix	No	1.30	0.74	1.90	2.50	Unexpected signal
<b>Test Track Driving (Drivers were aware of being observed)</b>								
Olson and Sivak, 1986	49	Young	No	1.10	0.15	1.35	1.60	Unexpected object
Olson and Sivak, 1986	15	Old	No	1.06	0.10	1.40	1.50	Unexpected object
Lerner et al., 1995	56	Mix	No	1.51	0.40	1.91	2.20	Unexpected object
Fambro et al., 1997	38	Mix	No	0.99	0.22	/	/	Unexpected object
Fitch et al., 2010	64	Mix	No	0.96	0.19	/	/	Unexpected object
<b>Naturalistic Driving (Drivers were aware of being observed)</b>								
Dozza, 2013	472	Mix	No	1.30	1.03	/	/	Unexpected hazard
Dozza, 2013	472	Mix	Yes	1.55	1.08	/	/	Unexpected hazard
Dozza, 2013	472	Mix	Yes (some)	1.45	1.07	/	/	Unexpected hazard
Gao and Davis, 2017	103	Mix	No	1.58	1.26	/	/	Unexpected hazard
Gao and Davis, 2017	103	Mix	Yes	2.11	1.36	/	/	Unexpected hazard
Cai and Savolainen, 2020	159	Mix	Mix	1.51	1.24	2.61	8.44	Unexpected hazard
<b>Alerted Driver (Expected Event)</b>								
<b>Test Driving (Drivers were aware of being observed)</b>								
Olson, Sivak, 1985	49	Young	No	0.72	0.11	0.95	1.11	Anticipated object
Olson, Sivak, 1985	15	Old	No	0.73	0.10	1.00	1.29	Anticipated object
Fambro et al., 1997	26	Mix	No	0.59	0.19	/	/	Anticipated object
Fitch et al., 2010	64	Mix	No	0.78	0.03	/	/	Anticipated barricade
Fitch et al., 2010	64	Mix	No	0.55	0.02	/	/	Anticipated auditory alarm

## SHRP2 Naturalistic Driving Study (NDS): Investigating Reaction Times and Deceleration Rates

- Largest NDS to date:
  - 6 geographic areas
  - 3,400+ drivers/vehicles
  - 5,400,000+ trips
  - 1800+ crashes
  - ~7000 near-crashes



- Roadway Information Database (RID)
  - 12,500+ miles of roadway information
    - Horizontal and vertical alignment
    - Cross-sectional characteristics
    - Historical data from DOTs



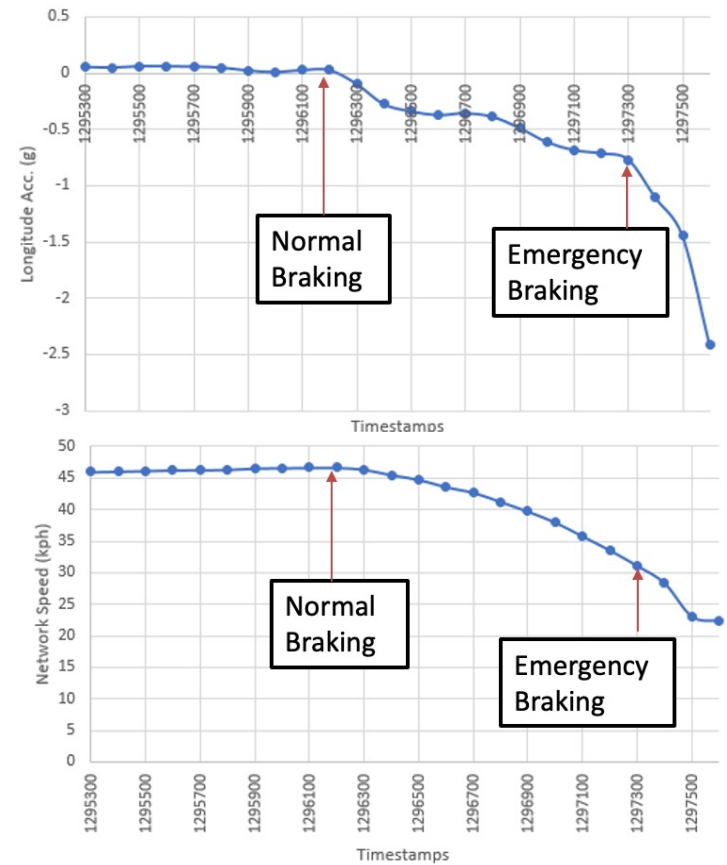
# Sample Event



1284244

VTI data exported for HRI EU  
<http://doi.org/10.15787/VTI1H590> 2017-11-17T17:25:50

Event Start = 1295300;  
 Onset of Normal Braking = 1296200;  
 Onset of Emergency Braking = 1297300;  
 Impact Time = 1297600;





# NDS Contextual Information



Rural



Rural Town



Suburban



Urban



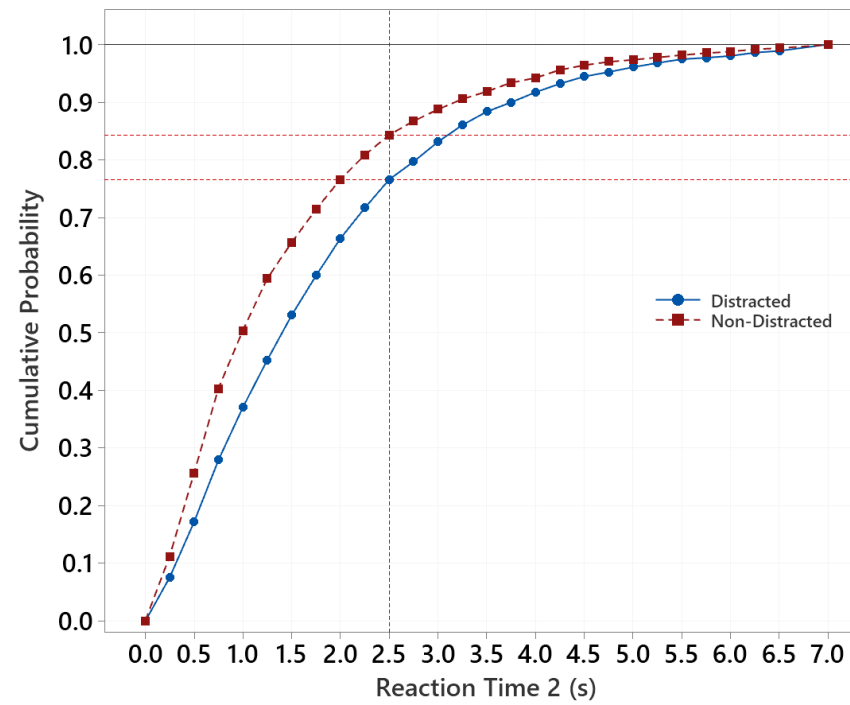
Urban Core



Sample Screenshots of Forward-View Video



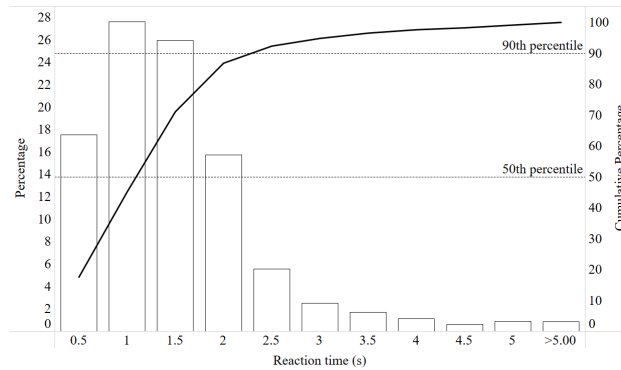
## NDS Reaction Time Data



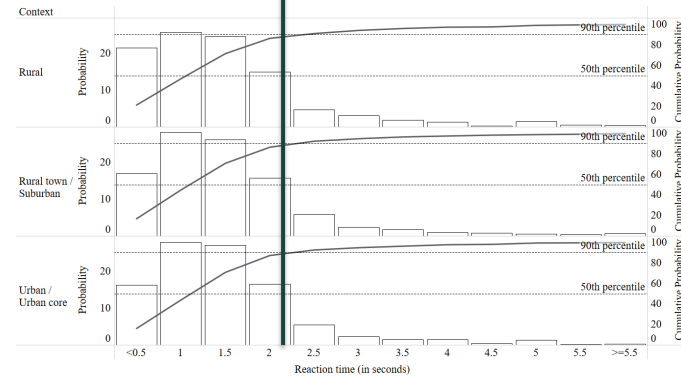
## NDS Reaction Time Results

Scenario	Reaction Time (s)	
	Mean	Std. Dev.
NCHRP Report 400 (Fambro et al., 1997)	1.140	0.204
SHRP 2 NDS – No secondary task events	1.120	0.884
SHRP 2 NDS – All safety-critical events	1.255	0.932
SHRP 2 NDS – Only secondary task events	1.332	0.950

90<sup>th</sup> percentile reaction time = 2.2 s  
(across all contexts)



Legend  
 ■ Cumulative Percentage  
 □ Percentage

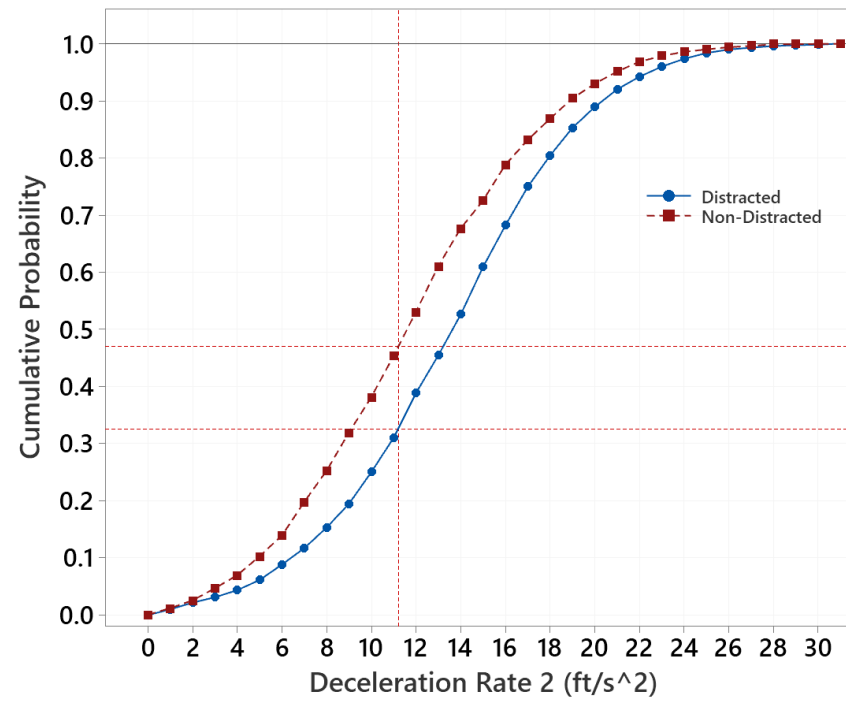


Legend  
 ■ Cumulative Probability  
 □ Probability

## Summary of Deceleration Rate Research

<b>Unsuspecting Driver (Unexpected Event, Unknown Time and Location)</b>					
	<b>Pavement/ Wheel Condition</b>	<b>Tangent/ Curve</b>	<b>Mean (g)</b>	<b>Std. Dev (g)</b>	<b>Stimulus</b>
<b>Test Track Driving (Drivers were aware of being observed)</b>					
Fambro et al., 1997	Dry/ABS	Tangent	0.63	0.08	Unexpected object
Fambro et al., 1997	Dry/No ABS	Tangent	0.62	0.08	Unexpected object
Fitch et al., 2010	Dry	Tangent	0.48	0.03	Unexpected barricade
Paquette and Porter, 2014	Dry	Tangent	0.82	0.27-0.67	Unexpected Signal
<b>Naturalistic Driving (Drivers were aware of being observed, but under real environment)</b>					
Wood, Zhang, 2017	Mix	Mix	0.44	0.26	Unexpected hazard
Lindheimer et al., 2018	Mix	Mix	0.26		Unexpected hazard
Savolainen et al., 2021	Mix	Mix	0.40	0.17	Unexpected hazard
<b>Alerted Driver (Expected Event, Unknown Time and Location)</b>					
<b>Test Track Driving (Drivers were aware of being observed)</b>					
Fambro et al., 1997	Dry/No ABS	Curve	0.54	0.20	Anticipated object
Fambro et al., 1997	Dry/No ABS	Tangent	0.53	0.08	Anticipated object
Fambro et al., 1997	Wet/No ABS	Curve	0.45	0.04	Anticipated object
Fambro et al., 1997	Wet/No ABS	Tangent	0.49	0.04	Anticipated object
Fitch et al., 2010	Dry	Tangent	0.44	0.02	Anticipated barricade
Fitch et al., 2010	Dry	Tangent	0.63	0.01	Anticipated alarm
EI-Shawarby et al., 2007	Dry	Tangent	0.22-0.60		Anticipated signal
Mean Estimates			0.51	0.07	

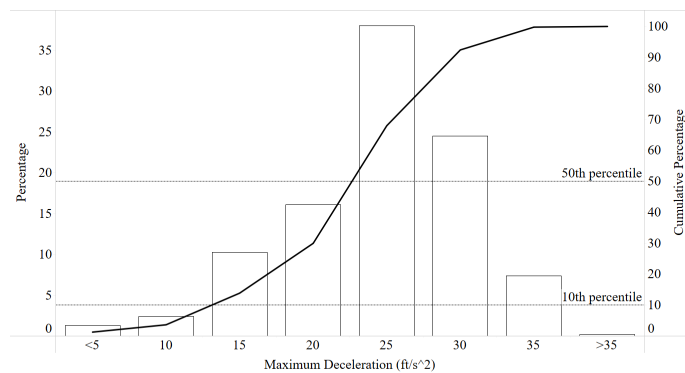
## NDS Deceleration Rate Data



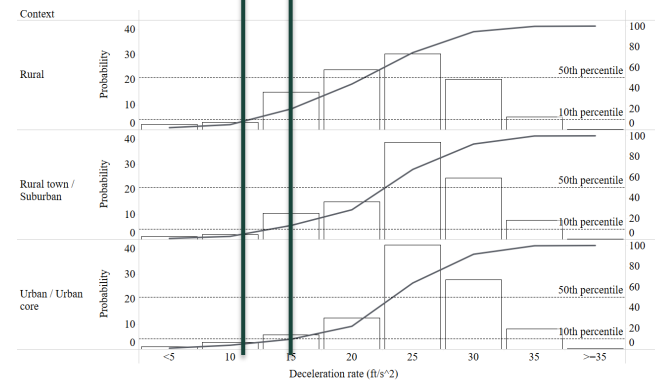
## NDS Deceleration Rate Results

Scenario	Deceleration Rate (ft/s <sup>2</sup> )	
	Mean	Std. Dev.
NCHRP Report 400 (Fambro et al., 1997)	29.302	4.508
SHRP 2 NDS – No secondary task events	20.707	6.269
SHRP 2 NDS – All safety-critical events	21.996	6.078
SHRP 2 NDS – Only secondary task events	22.727	5.843

10<sup>th</sup> percentile reaction times:  
 Rural setting = 11.8 ft/s<sup>2</sup>  
 Urban setting = 15.0 ft/s<sup>2</sup>



Legend  
 ■ Cumulative Percentage  
 □ Percentage



Legend  
 ■ Cumulative Probability  
 □ Probability

## Summary – SHRP 2 NDS Studies

- Perception-reaction time
  - Mean and 90th-percentile reaction times were 1.3 s and 2.2 s, respectively.
- Deceleration rate
  - 10th-percentile and average deceleration rates were 13.4 ft/s<sup>2</sup> and 22.0 ft/s<sup>2</sup>, respectively.
  - Rates were lower in higher-speed contexts (e.g., rural areas), where the 10th-percentile and average deceleration rates were 11.8 ft/s<sup>2</sup> and 20.4 ft/s<sup>2</sup>, respectively.
  - Rates were higher in lower speed contexts (e.g., urban areas), where the 10th-percentile and average deceleration rates were 15.0 ft/s<sup>2</sup> and 22.8 ft/s<sup>2</sup>, respectively.

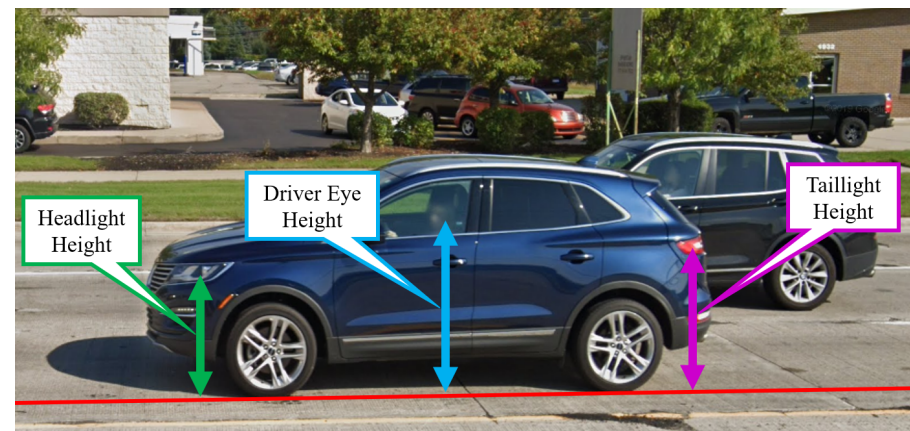
## Evaluation of Characteristics of Vehicle Fleet





## Data Collection – Driver Eye Height and Vehicle Headlight/Taillight Height

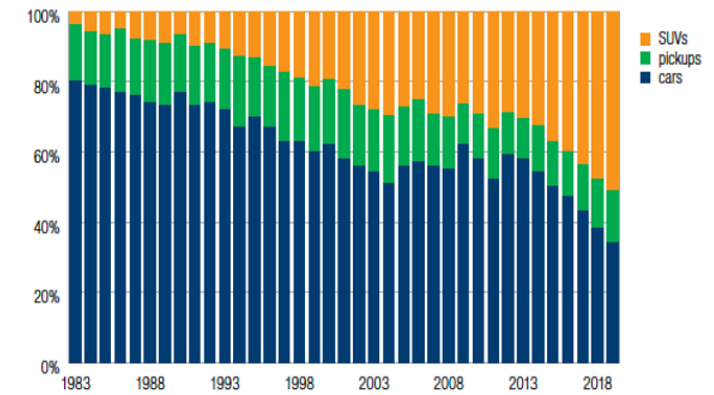
- Vehicle measurements obtained through direct measurement of parked vehicles.
- Driver eye height estimated to center of vehicle headrest.
  - Accuracy of  $\sim 0.1$  ft



## Comparison with NCHRP Report 400 – Passenger Vehicles

Descriptive Statistics	Headlight Height				Taillight Height			
	Passenger Cars		Multipurpose Vehicles		Passenger Cars		Multipurpose Vehicles	
	Present Study	NCHRP-400	Present Study	NCHRP-400	Present Study	NCHRP-400	Present Study	NCHRP-400
Sample Size	1,172	1318	1,442	992	1,172	858	1,442	534
Mean (ft)	2.31	2.13	3.00	2.76	2.97	2.38	3.57	3.16
10 <sup>th</sup> Percentile (ft)	2.12	1.98	2.66	2.34	2.74	2.11	3.19	2.68

Descriptive Statistics	Center of Headrest and Driver Eye Height			
	Passenger Cars		Multipurpose Vehicles	
	Present Study	NCHRP-400	Present Study	NCHRP-400
Sample Size	1,172	875	1,442	629
Mean (ft)	3.86	3.77	4.59	4.86
10 <sup>th</sup> Percentile (ft)	3.62	3.55	4.23	4.28



Vehicle Type Distribution

## Summary of Parking Lot Studies

- Driver eye heights
  - 90% driver eye heights exceed 3.75 ft for all passenger vehicles (slight increase compared to NCHRP 400)
  - No significant change in truck driver eye height
- Headlight height
  - No significant change compared to NCHRP 400
- Taillight height
  - Increased by 0.5 ft for passenger cars and multipurpose vehicles compared to NCHRP 400
  - However, no change is recommended to object height

# Recommended Changes to 2018 AASHTO Green Book



## Guidelines Related to SSD

- It is recommended to update the brake reaction time and deceleration rate values as follows:
  - Update brake reaction time from 2.5 s to 2.2 s
  - Deceleration rate to be updated to 11.8 ft/s<sup>2</sup> in rural contexts or high speed contexts (greater than 45 mph)
  - Deceleration rates to be updated to 15.0 ft/s<sup>2</sup> in urban and urban core context or low speed contexts (less than or equal to 45 mph)

## Guidelines Related to SSD

Proposed Table 3-1: Stopping Sight Distance on Level Roadways

### Rural or High Speed

U.S. Customary				
Design Speed (mph)	Brake Reaction Distance (ft)	Braking Distance on Level (ft)	Stopping Sight Distance Calculated (ft)	Stopping Sight Distance Design (ft)
15	48.5	20.5	69.0	70
20	64.7	36.4	101.1	105
25	80.9	56.9	137.8	140
30	97.0	82.0	179.0	180
35	113.2	111.6	224.8	225
40	129.4	145.8	275.1	280
45	145.5	184.5	330.0	335
50	161.7	227.8	389.5	390
55	177.9	275.6	453.5	455
60	194.0	328.0	522.0	525
65	210.2	384.9	595.1	600
70	226.4	446.4	672.8	675
75	242.6	512.4	755.0	760
80	258.7	583.1	841.8	845
85	274.9	658.2	933.1	935

### Low Speed Urban

U.S. Customary				
Design Speed (mph)	Brake Reaction Distance (ft)	Braking Distance on Level (ft)	Stopping Sight Distance Calculated (ft)	Stopping Sight Distance Design (ft)
15	48.5	16.1	64.6	65
20	64.7	28.7	93.3	95
25	80.9	44.8	125.6	130
30	97.0	64.5	161.5	165
35	113.2	87.8	201.0	205
40	129.4	114.7	244.0	245
45	145.5	145.1	290.7	295

## Guidelines Related to SSD

- It is recommended to update the criteria for measuring SSD as follows:
  - Driver's eye height be increased from 3.50 ft to 3.75 ft.
  - No change in truck driver's eye height (7.6 ft).
  - Object height for SSD scenarios should remain the same (2 ft).
- These updates will also result in updating object height criteria for passing sight distance (PSD) and intersection sight distance to 3.75 ft

## Guidelines Related to Crest Vertical Curves

- Following updates to design parameters are recommended:
  - Eye height should be increased to 3.75 ft.
  - Object height should remain at 2.0 ft.
- These updates will result in revised design controls for crest vertical curves based on SSD and PSD, i.e., revised values for rate of vertical curvature ( $K_a$ ).



## Guidelines Related to Sight Distance at Undercrossings

- Following updates to design parameters are recommended:
  - Eye height should be changed to 7.6 ft for truck eye height
  - Object height should be increased to 3.0 ft for taillights of a vehicle



**But.....**

**What are the impacts of SSD on crash risk?**



## The Relationship between SSD and Crash Risk

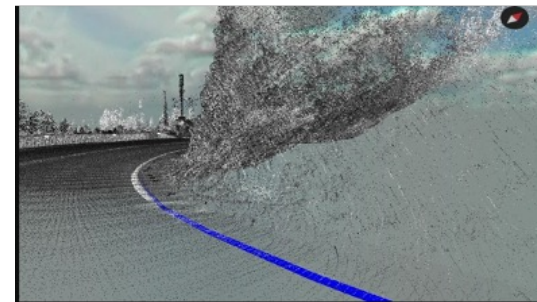
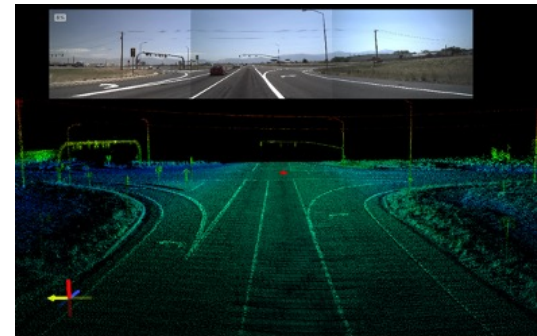
- SSD is one of the ten controlling criteria for design and documentation of design exceptions. Applicable for horizontal and vertical alignments, except for sag vertical curves.
- The extant research literature has generally not shown that locations with insufficient SSD experience higher crash risks across most contexts.
- An exception is crest vertical curves on two-lane highways where a hidden curve, intersection, ramp, or driveway is present [Fambro et al., 1997; Harwood et al., 2014].
- Why?

## Factor of Safety Inherent in SSD Model

- Conservative design assumptions for each parameter (e.g., crest vertical curve case):
  - 90<sup>th</sup> percentile brake reaction time (2.5 s)
  - 90<sup>th</sup> percentile speed (design speed)
  - 10<sup>th</sup> percentile deceleration rate (11.2 ft/s<sup>2</sup>)
  - 10<sup>th</sup> percentile driver eye height (3.5 ft)
- Assuming independence, probability of conditions being met simultaneously = 0.0001.
- Probability of conditions being met AND a stopping scenario arising  $\ll$  0.0001.
- SSD has also been resource-intensive to measure historically.

## SSD – Safety Analysis

- High-fidelity LiDAR data were utilized to assess relationship between available sight distance and crashes.
- Data from the Utah DOT were analyzed for freeways (80 mph speed limit) and two-lane highways (speed limits ranging from 30 mph to 65 mph).
  - Follow-up work in Michigan on two-lane highways posted at 55 mph or 65 mph.



Mileage : 9.7097  
Visibility : 50%  
Tested Speed : 55 mph  
Max Speed : 55 mph

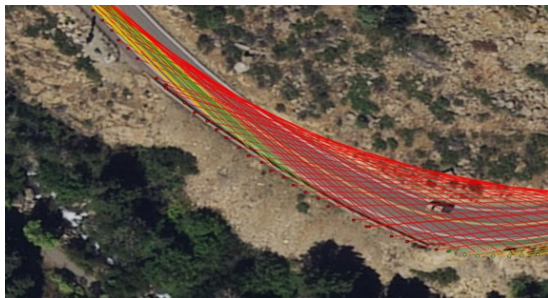


Mileage : 8.3653  
Visibility : 20%  
Tested Speed : 55 mph  
Max Speed : 70 mph



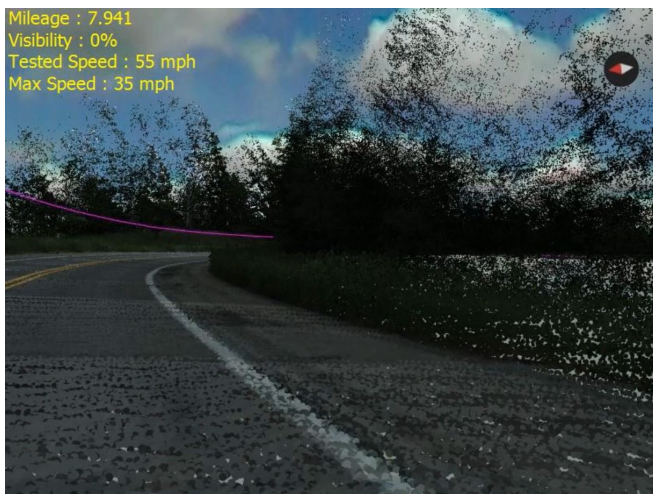


# Sample Output from LIDAR Tool

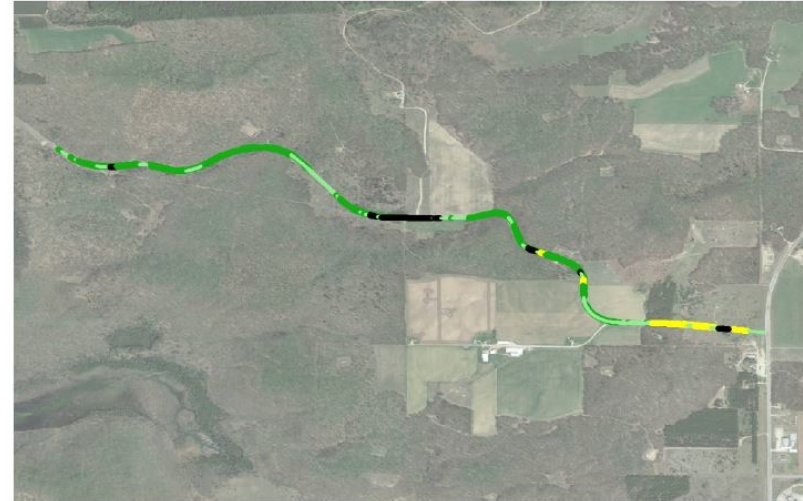


ID	RName	Direction	PathCode	PathID	ConfType	Mileage	TSpeed	MaxSpeed	Slope	Visibility	Cycle	Classify	Action
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1514	40	35	-4.569184303	6.666666985	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1533	40	30	-5.955739498	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1552	40	30	-4.998222828	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1571	40	30	-5.361332417	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.159	40	30	-5.660354137	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1609	40	30	-5.131266117	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1628	40	30	-4.799786568	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1646	40	35	-4.301214218	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1665	40	35	-3.8682127	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1684	40	35	-3.739371777	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1703	40	35	-4.498730659	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1722	40	35	-3.274622917	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1741	40	35	-4.10327816	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.176	40	35	-3.472764015	0	2017	Terrain	Engineering redesign
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1779	40	35	-3.408043623	1.769911528	2017	Vegetation	Maintenance vegetation
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1798	40	40	-3.969248295	66.66667175	2017		
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1817	40	40	-3.573226929	92.10526276	2017		
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1836	40	40	-3.108936787	92.98246002	2017		
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1855	40	40	-3.404061556	92.10526276	2017		
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1874	40	40	-3.937189817	93.85964966	2017		
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1893	40	40	-4.070704937	88.59649658	2017		
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1912	40	45	-4.434724808	92.10526276	2017		
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1931	40	45	-4.002924919	88.59649658	2017		
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1949	40	45	-4.763065815	90.56604004	2017		
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1968	40	45	-4.867936611	91.50942993	2017		
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.1987	40	45	-5.094794273	90.47618866	2017		
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.2006	40	30	-5.3622756	0.952381015	2017	Vegetation	Maintenance vegetation
9/2/21 13:33	190	Negative	CL	190-CL-N	SSD	3.2025	40	30	-5.362917423	0	2017	Vegetation	Maintenance vegetation





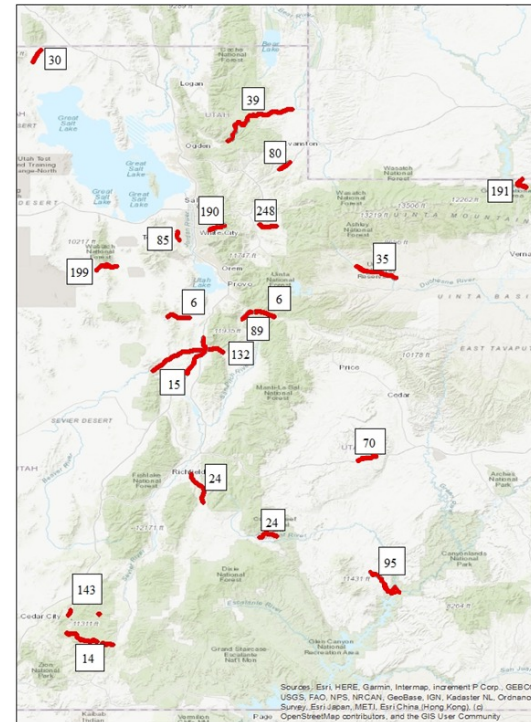
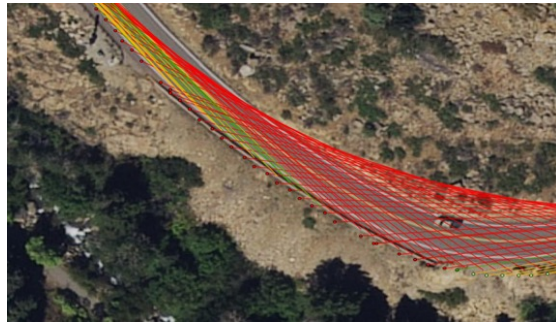
Stopping Sight Distance - By Classification



Obstacle types (count)

- SSD Position Classification
  - MICHIGAN\_0032-1-LL-P\_SSD\_Position
    - Vegetation [499]
    - Vertical curvature [96]
    - Vehicles [135]
    - Visible [477]
  - MICHIGAN\_0032-1-LL-N\_SSD\_Position
    - Vegetation [632]
    - Vertical curvature [79]
    - Visible [498]

# SSD – Safety Analysis for Utah Data

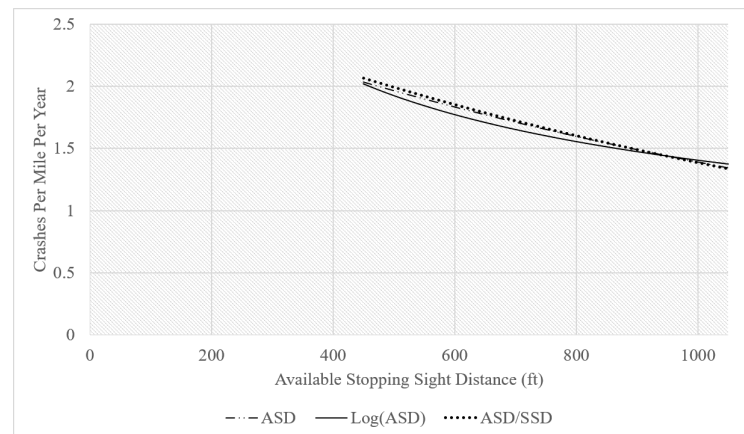
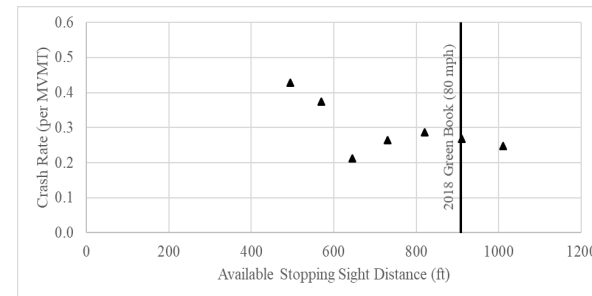


Variable	Freeway		Non-Freeway	
	Number	Percent	Number	Percent
<b>Crashes by Severity Level</b>				
K	2	0.6	19	1.1
A	8	2.6	83	5.0
B	22	7.1	200	12.1
C	33	10.6	182	11.0
O	246	79.1	1,174	70.8
<b>TOTAL</b>	<b>311</b>	<b>100.0</b>	<b>1,658</b>	<b>100.0</b>
<b>Manner of Collision</b>				
Not Applicable/Single Vehicle	238	76.5	1,218	73.5
Front to Rear	25	8.0	164	9.9
Sideswipe Same Direction	39	12.5	64	3.9
Sideswipe Opposite Direction	0	0.0	61	3.7
Parked Vehicle	5	1.6	29	1.7
Head On (front-to-front)	0	0.0	19	1.1
Angle	4	1.3	99	6.0
<b>TOTAL</b>	<b>311</b>	<b>100.0</b>	<b>1,658</b>	<b>100.0</b>



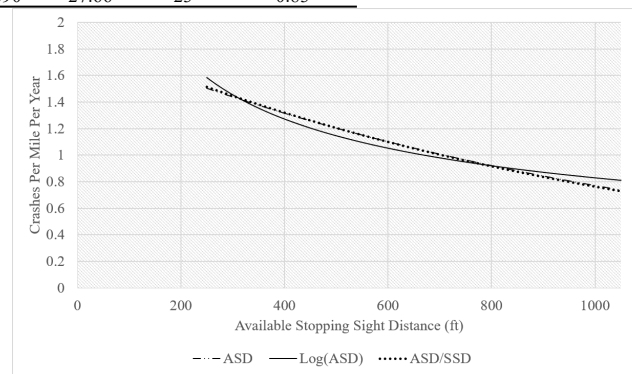
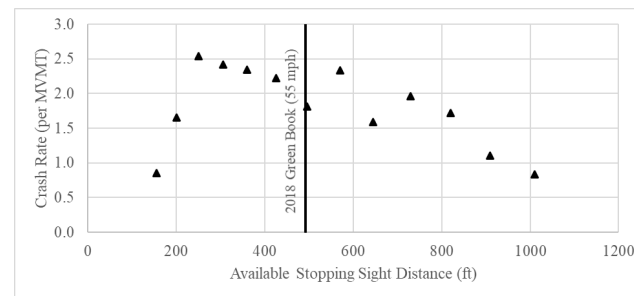
# Freeway (Utah)

Minimum Available SSD (ft)	No. of Segments	No. of Miles	Avg. AADT	Total MVMT	Total Crashes	Crash Rate per MVMT
≤495	72	7.14	18702	242.76	104	0.43
570	13	1.26	17468	40.02	15	0.37
645	34	3.41	15550	94.65	20	0.21
730	37	3.57	18146	117.17	31	0.26
820	54	5.17	15952	149.54	43	0.29
910	26	2.52	14749	67.36	18	0.27
1010	132	12.46	14392	322.37	80	0.25



## Non-Freeway (Utah)

Minimum Available SSD (ft)	No. of Segments	No. of Miles	Avg. AADT	Total MVMT	Total Crashes	Crash Rate per MVMT
≤155	53	5.01	1,054	9.41	8	0.85
200	56	5.28	881	8.45	14	1.66
250	95	8.84	1,237	19.31	49	2.54
305	172	16.40	1,102	31.82	77	2.42
360	164	15.70	831	23.03	54	2.34
425	265	25.66	683	32.00	71	2.22
495	82	7.85	884	12.14	22	1.81
570	84	8.27	842	12.85	30	2.33
645	59	5.34	911	8.82	14	1.59
730	51	4.69	821	6.61	13	1.97
820	46	4.61	862	6.99	12	1.72
910	55	5.32	1,083	10.87	12	1.10
1010	95	8.97	1,690	27.66	23	0.83





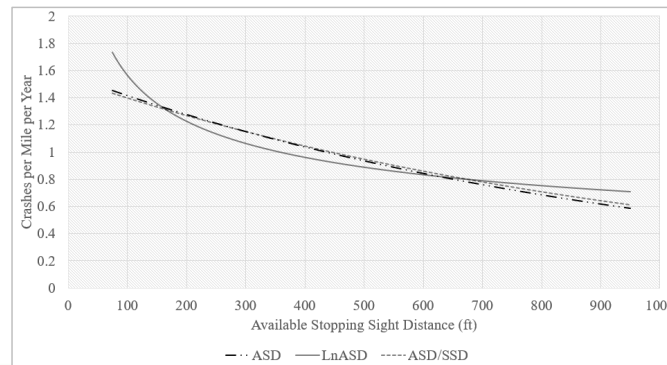
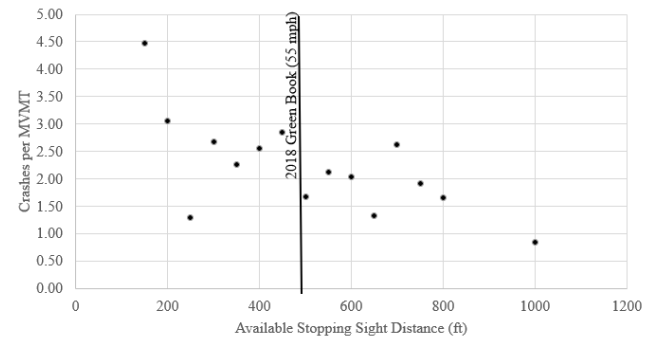
## SSD – Safety Analysis for Michigan Data

- Analysis carried out for smaller road network.
- 21 miles of rural two-lane highways posted at 55/65 mph were identified
  - Frequent occurrence of curvature
  - Varying traffic volume and geometry



## Non-Freeway (Michigan)

Minimum Available SSD (ft)	No. of Segments	No. of Miles	Avg. AADT	Total MVMT	Total Crashes	Crash Rate per MVMT
<150	30	3.06	1,793	10.04	40	4.47
200	28	2.83	1,737	9.04	26	3.06
250	17	1.76	1,562	4.99	6	1.29
300	16	1.60	1,817	5.31	14	2.68
350	16	1.60	1,680	4.91	11	2.26
400	10	1.00	1,931	3.52	9	2.56
450	16	1.60	2,282	6.66	18	2.86
500	12	1.20	2,148	4.70	9	1.67
550	7	0.73	2,015	2.69	6	2.12
600	11	1.10	2,332	4.68	9	2.04
650	12	1.20	2,365	5.18	7	1.33
700	5	0.50	2,130	1.94	5	2.62
750	7	0.77	2,288	3.22	7	1.92
800	11	1.10	2,280	4.58	7	1.66
1000	16	1.60	2,320	6.78	6	0.85



**The Future:  
What about Advanced Driver Assistance  
Systems (ADAS) and Automated Driving  
Systems (ADS)?**





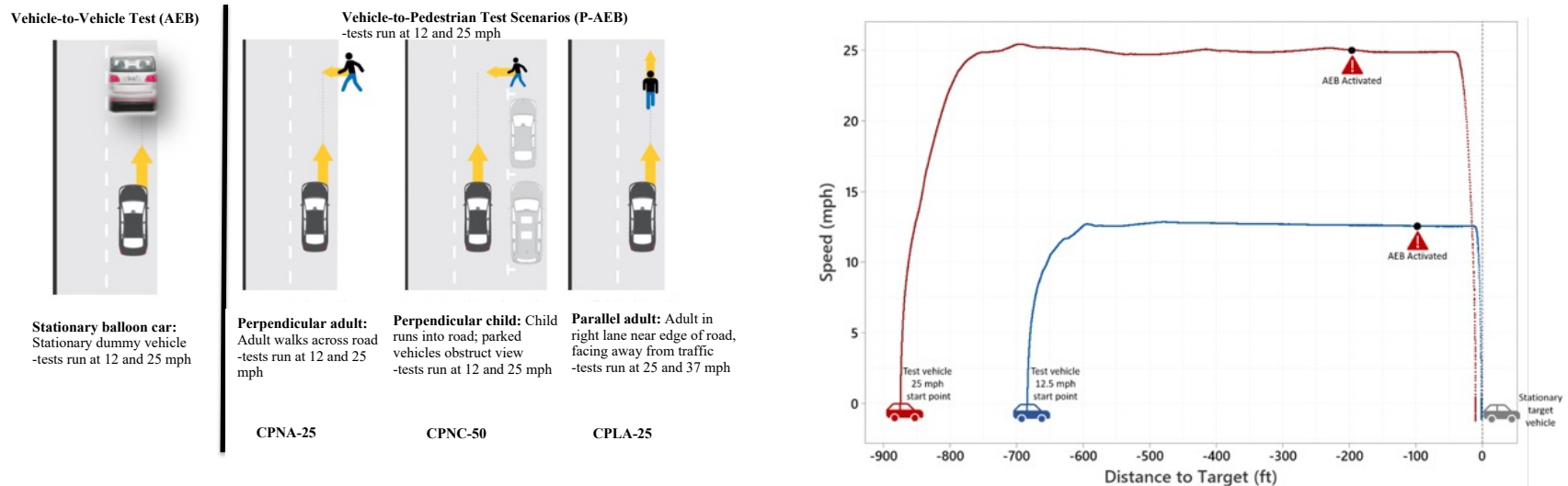
## SAE J3016™ LEVELS OF DRIVING AUTOMATION

	SAE LEVEL 0	SAE LEVEL 1	SAE LEVEL 2	SAE LEVEL 3	SAE LEVEL 4	SAE LEVEL 5
What does the human in the driver's seat have to do?	You are driving whenever these driver support features are engaged – even if your feet are off the pedals and you are not steering			You are not driving when these automated driving features are engaged – even if you are seated in "the driver's seat"		
	You must constantly supervise these support features; you must steer, brake or accelerate as needed to maintain safety			When the feature requests, you must drive	These automated driving features will not require you to take over driving	
What do these features do?	These are driver support features			These are automated driving features		
	These features are limited to providing warnings and momentary assistance	These features provide steering OR brake/acceleration support to the driver	These features provide steering AND brake/acceleration support to the driver	These features can drive the vehicle under limited conditions and will not operate unless all required conditions are met	This feature can drive the vehicle under all conditions	
Example Features	<ul style="list-style-type: none"> <li>• automatic emergency braking</li> <li>• blind spot warning</li> <li>• lane departure warning</li> </ul>	<ul style="list-style-type: none"> <li>• lane centering OR</li> <li>• adaptive cruise control</li> </ul>	<ul style="list-style-type: none"> <li>• lane centering AND</li> <li>• adaptive cruise control at the same time</li> </ul>	<ul style="list-style-type: none"> <li>• traffic jam chauffeur</li> </ul>	<ul style="list-style-type: none"> <li>• local driverless taxi</li> <li>• pedals/steering wheel may or may not be installed</li> </ul>	<ul style="list-style-type: none"> <li>• same as level 4, but feature can drive everywhere in all conditions</li> </ul>



## Analysis of Automatic Emergency Braking (AEB) Data

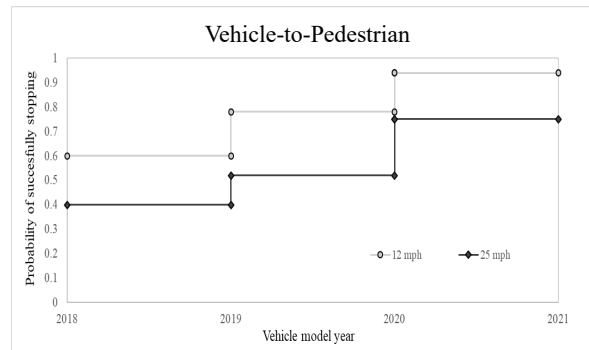
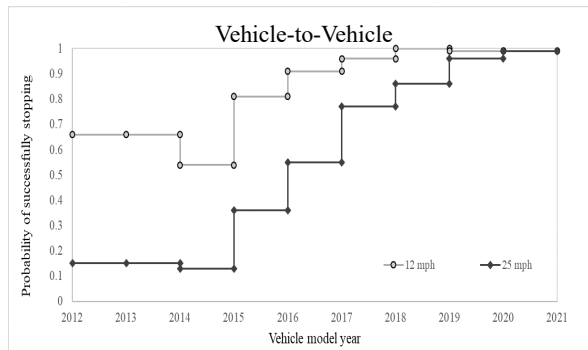
- Data from the Insurance Institute for Highway Safety (IIHS) were obtained for automatic emergency braking (AEB) systems.



Source: HLDI, 2019

## AEB Test Results

Test Type	Test Speed (mph)	Sample Size	Success Rate (%)	Avg. Speed Reduction (mph)	Avg. FCW TTC(s)	Avg. AEB TTC (s)	Max. Decel. Rate (ft/s <sup>2</sup> )
AEB	12	1323	87.0	11.6	1.4	0.8	27.1
AEB	25	1273	62.4	19.0	2.1	1.1	27.1
P-AEB	12	400	88.0	18.1	1.1	0.7	29.6
P-AEB	25	400	75.8	34.4	1.3	0.9	30.1
P-AEB	12	402	80.3	16.9	1.0	0.7	27.8
P-AEB	25	401	48.6	27.9	0.9	0.7	29.6
P-AEB	25	400	82.3	21.8	1.7	1.2	29.0
P-AEB	37	400	34.0	25.2	1.7	1.2	28.9



# Integration of Advanced Driver Assistance Systems (ADAS) into New Vehicles

Figure 6: Proportion of vehicle series with forward collision warning, 2000–20 model years

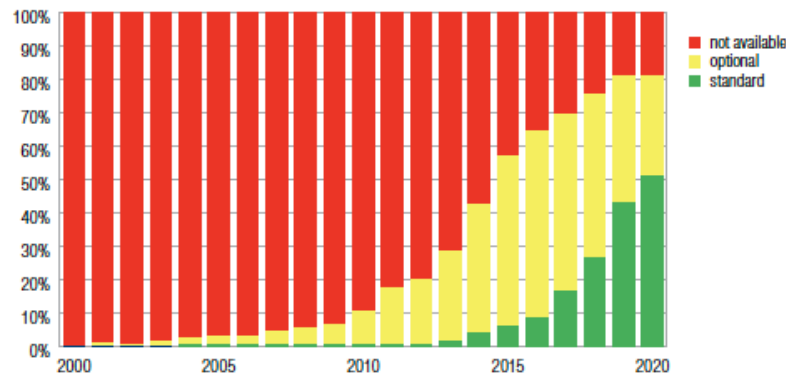
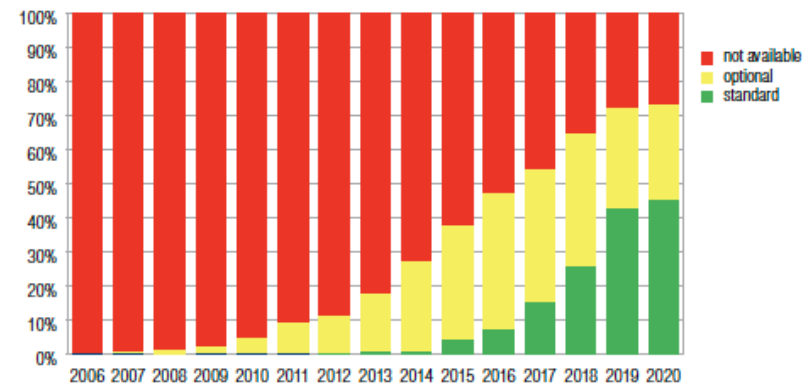


Figure 7: Proportion of vehicle series with forward collision warning with autobrake, 2006–20 model years



# Fleet Penetration for Forward Collision Warning and Automatic Emergency Braking

**Figure 8: Percentage of registered vehicles with front crash prevention by calendar year**

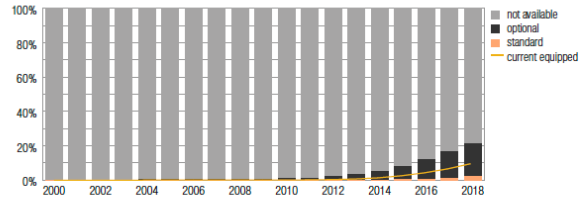


Figure 8 shows the percentage of registered vehicles by calendar year with either standard or optional front crash prevention. In 2006, front crash prevention had become standard on less than 1 percent and optional on less than 1 percent of registered vehicles. By 2018, front crash prevention was standard or optional on 21 percent of registered vehicles, with about 10 percent of registered vehicles estimated to be equipped with the feature.

**Figure 11: Percentage of registered vehicles with front automatic emergency braking by calendar year**

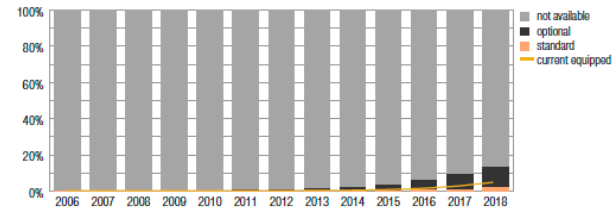


Figure 11 shows the percentage of registered vehicles by calendar year with either standard or optional front AEB. In 2012, AEB had become standard on less than 1 percent and optional on 1 percent of registered vehicles. By 2018, AEB was standard or optional on 13 percent of registered vehicles but estimated to be equipped only on 5 percent.

**Figure 9: Predicted percentage of registered vehicles with front crash prevention by calendar year**

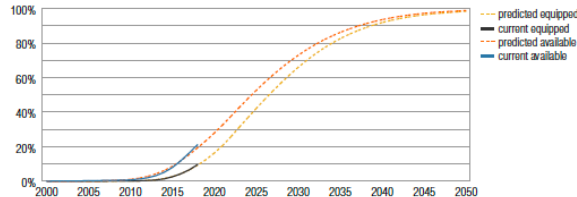


Figure 9 takes into account a voluntary commitment by many manufacturers to make front AEB standard on most of their vehicles by 2022. It shows the predicted registered vehicles by calendar year with front crash prevention. One prediction is for vehicles with front crash prevention available (standard or optional) and the other prediction is for vehicles equipped (standard or optionally equipped) with front crash prevention. It is predicted that 95 percent of registered vehicles will be equipped with the feature in 2043.

**Figure 12: Predicted percentage of registered vehicles with front automatic emergency braking by calendar year**

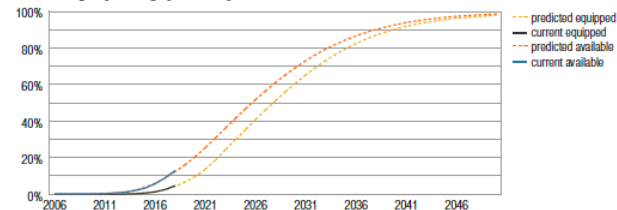


Figure 12 takes into account the 2022 voluntary commitment and shows the predicted registered vehicles by calendar year with front AEB. One prediction is for vehicles with AEB available (standard or optional) and the other prediction is for vehicles equipped (standard or optionally equipped) with AEB. It is predicted that 95 percent of registered vehicles will be equipped with AEB in 2044.

## Areas for Future Research

- Additional investigation into the relationship between crash risk and available stopping sight distance (e.g., crash modification functions).
- Probabilistic design in consideration of the distributions of reaction time, deceleration rate, and crash risk.
- Incorporation of the effects of automatic emergency braking systems into design criteria.

**Thank You!**

**Questions?  
Comments?**




Peter T. Savolainen, Ph.D., P.E.  
MSU Research Foundation Professor and Chairperson  
Department of Civil and Environmental Engineering  
Michigan State University  
428 S Shaw Ln, Room 3546C  
East Lansing, MI 48824  
Phone: (517) 432-1825  
E-mail: [pete@msu.edu](mailto:pete@msu.edu)

**Michigan State University**  
**Department of Civil and Environmental Engineering:**  
**An Overview**

**Presentation to National Technical University of Athens (NTUA)**  
**November 11, 2024**

Peter T. Savolainen, Ph.D., P.E.  
MSU Research Foundation Professor & Chairperson  
Department of Civil & Environmental Engineering  
Michigan State University



## Introduction

- MSU
  - founded in 1855, first land-grant institution
  - > 50,000 students
  - 5,300 faculty members and academic staff
  - 17 colleges, 200 degree programs
- College of Engineering
  - 6000 undergraduate, 1000 graduate students
  - 200 faculty members
  - 8 departments, 9 graduate/10 undergraduate degree programs

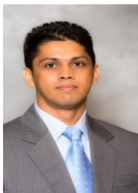
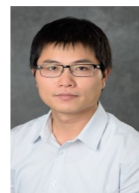


## Introduction

- Department of Civil and Environmental Engineering
  - BS, MS and PhD degrees in Civil Engineering and Environmental Engineering
  - 31 core faculty members
  - 600 undergraduate students
  - 130 graduate students
  - 43 undergraduate courses and 36 graduate courses offered
  - > 50 active research projects
  - > \$11 million annual research expenditures

Civil and Environmental Engineering Faculty

MICHIGAN STATE UNIVERSITY



## Research Specialty Areas

- Renewable Energy, Environmental Conservation, Recycled Materials
- Public Health, Water Quality, Microbiology, Environmental Chemistry
- Design, Construction, and Environmental Sustainability of Geotechnical and Pavement Systems
- Hydrology, Climate Change, and Water Resource Systems Modeling
- Structural Engineering, Mechanics, and Materials
- Transportation Safety, Human Factors, Travel Demand Modeling, Connected/Automated Vehicles, Sustainable Transportation

## Graduate Degree Programs

- MS – 30 credits
  - Option A – 24 course credits; 6 thesis credits
  - Option B – 30 course credits
- PhD – 36-48 credits
  - 12 course credits
  - 24-36 dissertation credits
- Dual PhD – TBD